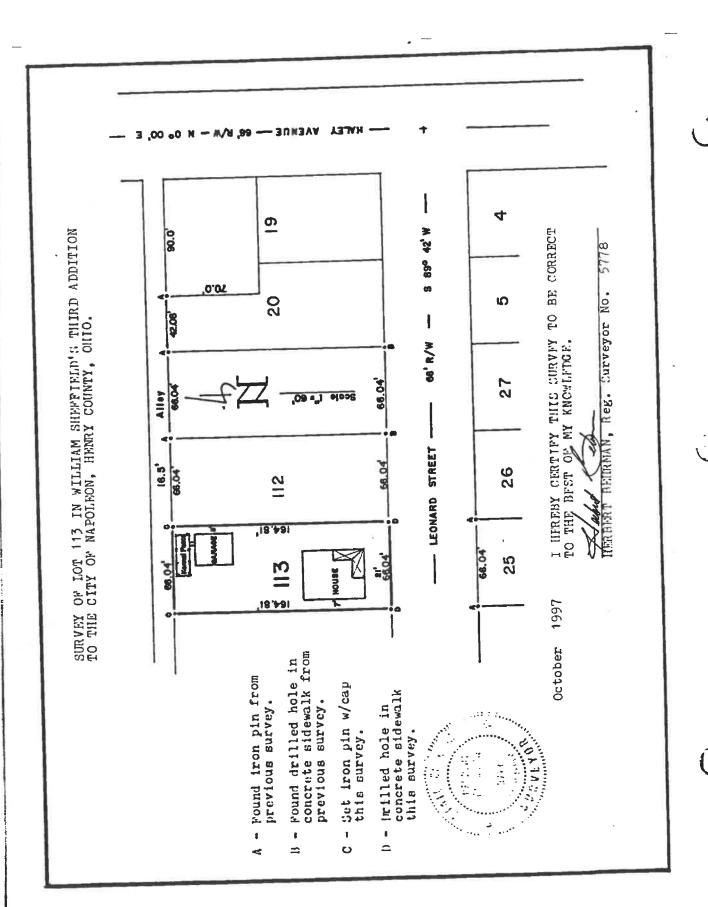
CITY OF NAPOLEON GENERAL PERMIT APPLICATION

THIS APPLICATION IS FOR RESIDENTAL CONSTRUCTION INCLUDING BUILDING, ELECTRICAL, PLUMBING, MECHANICAL & REMODELING

	rL(JMBING, MECHANICAL &	REMODELING	,	
DATE 8-5-2014	_ JOB LOCATION _	632 Leona		Napol	250
OWNER Dean	3.110		TE		# 419-966-1227
OWNER ADDRESS	532 Leonard	&. Nopola-	, oH 4	3545	
CONTRACTOR			CE	LL PHONE	#
DESCRIPTION OF WO	ORK TO BE PERFORM	MED Build	oarad		
ESTIMATED COMPL	ETION DATE	r 30,20,4 B	STIMATED C	osт <u></u> ≴ 7	CCC
Affected Floor Area (AFA only the room and not all the		s the area affected by the impro	ovement, i.e. a nev	v wall dividin	g a room (the AFA would be
DESCRIPTION				FEE	TOTAL COST
BUILDING:					
Decks				\$25.00	\$
Addition & Alteratio	ons Square foot in (AFA)	x \$0.05 =	+	\$25.00 =	\$
✓ Garage and Shed or	ver 200 SF (Detached)			\$25.00	\$
Siding and/or Roofi	ing			\$25.00	\$
Windows/Doors				\$25.00	\$
ELECTRICAL:					
Electrical	Circuits in (AFA)	x \$3.00/Circuit = \$	+	\$25.00 =	\$
Electrical Service U	pgrade			\$25.00	\$
MECHANICAL:					
Water Heater				\$25.00	\$
Furnace and/or AC	Replacement			\$25.00	\$
PLUMBING:					
Plumbing	Traps in (AFA)	x \$3.00/Trap = \$	+	\$25.00 =	\$
	ГОТ	AL plus Ohio Board of Bu	uilding Standard	ls Fee 1%	\$
			TO	TAL FEE:	¢.
ALTERATION OF ANY BUILDI	NG STRUCTURE, SIGN, OR PA	TION OR STRUCTURAL ALTERART THEREOF AND NO USE OF THE SELECTION OF NAPOL	ATION, ELECTRICA HE ABOVE SHALL I	L OR MECHA! BE UNDERTAK	EN OR PERFORMED UNTIL THE
application as his her authorized ag	ent and I agree to conform to all app	roposed work is authorized by the Own blicable laws of the jurisdiction. In add we the authority to enter areas covered	lition, if a permit for We	ork described in ti	his application is issued, I certify that
I HEREBY ACKNOWLEDG	E THAT I HAVE READ AND	FULLY UNDERSTAND THE	ABOVE LISTED 1	NSTRUCTIO	NS.
SIGNATURE OF APPLICANT	Den D	£	DATE:	8-5-	2014
PRINT NAME: >> €	ean Detme				
	ватсн# 30524	снеск # 1548	DATE_8	-5-14	





Page 1 of 3 7/27/2014

Items Selected:

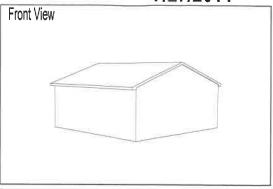
Biltmore, Dual Black Shingles 4' Shingleover Ridge Vent White Vinyl Soffit & Fascia White Regular Roof Edge Pine Overhead Door Jamb

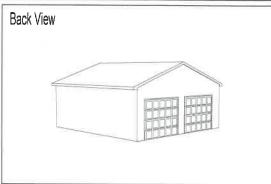
Gable roof w/ 4/12 pitch, standard trusses 2' O.C.
Truss Design Location Zip Code: 43545
Residential Post Frame Construction
with Concrete Floor (not included)
24' Wide X 24' Deep X 9' - 6" High
Vinyl Dbl 3.5" Lap Siding
- White
7/16" OSB Wall Sheathing
Block-It Housewrap
0" gable/12" eave overhangs
7/16" OSB Roof Sheathing

The options you have selected are: 15 LB Roof Felt

Options Selected:

1 - 9x7 Overhead Door - Insulated RP White





7/12 pitch 2-9/x7' Doors 1-32" Sid Door

Estimated base price: \$4,574.94* The base price includes: 0" Eave/0" Gable Overhangs, Framing Materials,

The base price includes: 0" Eave/0" Gable Overhangs, Framing Materials,
7/16 OSB Roof Sheathing, 20 yr. Fiberglass Classic - Onyx Black Shingles, Pine Fascia, Galv Regular Roof Edge,
8" Textured Vertical Hardboard Siding, All selected overhead, service and sliding doors are included.
Windows and other options are NOT included.

Estimated price: \$5,411.72*

*Today's estimated price, future pricing may go up or down.
*Tax, labor, and delivery not included.

*** Take this sheet to the Building Materials counter to purchase your materials. ***

Floor type (concrete, dirt, gravel) is NOT included in estimated price. The floor type is used in the calculation of materials needed. Labor, foundation, steel beams, paint, electrical, heating, plumbing, and delivery are also NOT included in estimated price. This is an estimate. It is only for general price information. This is not an offer and there can be no legally binding contract between the parties based on this estimate. The prices stated herein are subject to change depending upon the market conditions. The prices stated on this estimate are not firm for any time period unless specifically written otherwise on this form. The availability of materials is subject to inventory conditions. MENARDS IS NOT RESPONSIBLE FOR ANY LOSS INCURRED BY THE GUEST WHO RELIES ON PRICES SET FORTH HEREIN OR ON THE AVAILABILITY OF ANY MATERIALS STATED HEREIN.

All information on this form, other than price, has been provided by the guest and Menards is not responsible for any errors in the information on this estimate, including but not limited to quantity, dimension and quality.

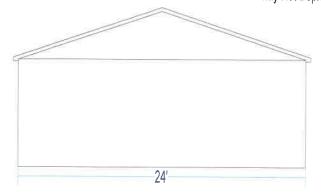
Please examine this estimate carefully. MENARDS MAKES NO REPRESENTATIONS, ORAL, WRITTEN OR OTHERWISE THAT THE MATERIALS LISTED ARE SUITABLE FOR ANY PURPOSE BEING CONSIDERED BY THE GUEST. BECAUSE OF THE WIDE VARIATIONS IN CODES, THERE ARE NO REPRESENTATIONS THAT THE MATERIALS LISTED HEREIN MEET YOUR CODE REQUIREMENTS. THE PLANS AND/OR DESIGNS PROVIDED ARE NOT ENGINEERED. LOCAL CODE OR ZONING REGULATIONS MAY REQUIRE SUCH STRUCTURES TO BE PROFESSIONALLY ENGINEERED AND CERTIFIED PRIOR TO CONSTRUCTION.

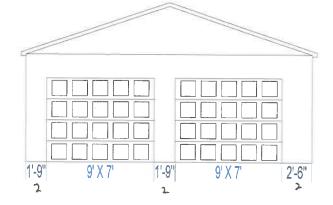


Page 2 of 3 7/27/2014

*** Here are the wall configurations for your design.

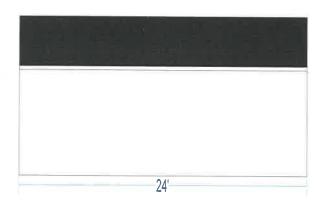
Illustration May Not Depict All Options Selected

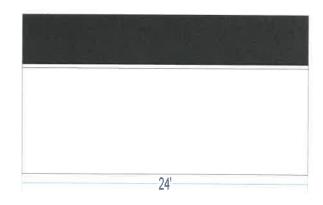




Gable Front View

(2) - 9X7 WHITE INSUL RAISEDPNLEZSETTORSN M4SV

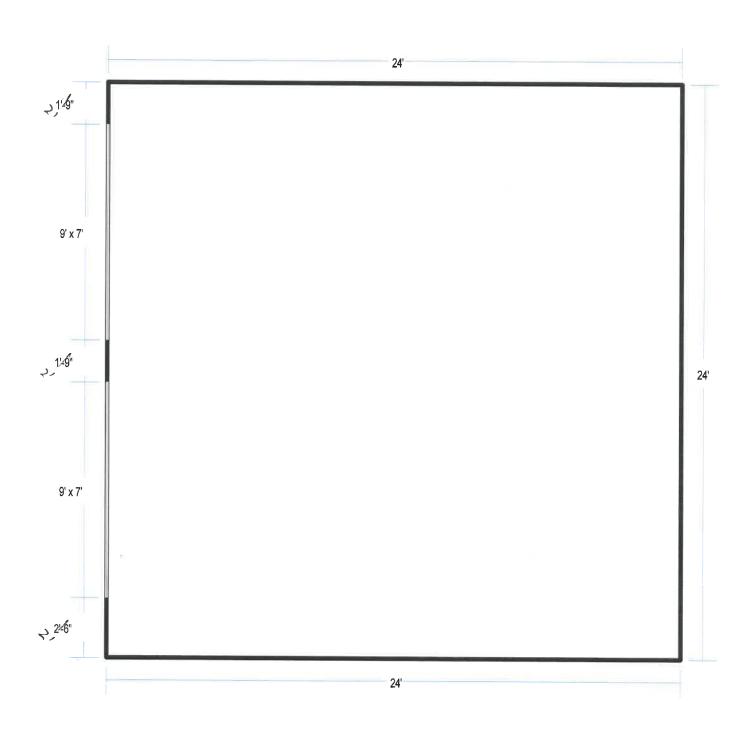




Eave Front View

Eave Back View

Building Size: 24 feet wide X 24 feet long X 9 feet high Approximate Peak Height: 13 feet 4 inches (160 inches) Illustration May Not Depict All Options Selected



Midwest Manutactuning, Eau Claire, WI 54/03

7.400 s Dec 26 2012 Mi Lek Industries, Inc. Sat Aug 03 12:36:59 2013 Hage 1 ID:wLq4tCSqiddmQCMKdSi1_CyrZ6B-HtgEfEXnxMKm56bmOpiDCarpbCFDKjkl6Dcx1RyrYz2



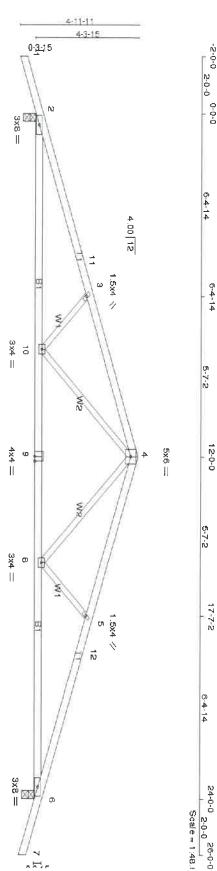
LOADING (psf)

0-0-0

15-8-12

8-3-4

24-0-0



REACTIONS (Ib/siz	2 X X X	TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2	TCDL 10.0 BCLL 0.0 BCDL 10.0	TCLL(roof) 28.1/5
(lb/size) 6=1317/0-3-8 Max Horz 2=-61(LC 19) Max Uplift6=-156(LC 11	onic all	PF No.2	10.0 0.0 *	42.0 /55.0
(lb/size) 6=1317/0-3-8 (min. 0-2-10), 2=1317/0-3-8 (min. 0-2-10) Max Horz 2=61(LC 19) Max Uplift6=-156(LC 11), 2=-156(LC 10)			Rep Stress Incr YES Code IRC2012/TPI2007	SPACING 2-0-0 Plates Increase 1.15
3-8 (min. 0-2-10)		m J =	<u> </u>	CSI TC 0.81
		BRACING TOP CHORD BOT CHORD	Horz(TL)	Vert(LL)
SI CHIA	MiTek re bracing I	Structural Rigid ceili	0.13	
	commence be installe	wood she	6 n/a	(loc) Vdefl 8-10 >999
wiii Stabilizer IIIstalialion guide.	s that Stabilized during truss	Structural wood sheathing directly applied or 2-2 Rigid ceiling directly applied or 2-2-0 oc bracing.		yfi
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance	Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing.	Weight: 74 lb FT =	PLATES MT20
	oss ance	purlins.	o FT=	GRIP 197/144

JOINT STRESS INDEX 2 = 0.79, 3 = 0.51, 4 = 0.78, 5 = 0.51, 6 = 0.79, 8 = 0.90, 9 = 0.87 and <math>10 = 0.90

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-11=-3428/328, 3-11=-3240/343, 3-4=-2969/294, 4-5=-2969/294, 5-12=-3240/343, 6-12=-3428/328

BOT CHORD 2-10=-243/3144, 9-10=-121/2160, 8-9=-121/2160, 6-8=-243/3144

WEBS 3-10=-736/156, 4-10=-23/953, 4-8=-24/953, 5-8=-736/156

Max Grav 6=1693(LC 3), 2=1693(LC 3)

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-10; Pr=42.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=55.0 psf (ground snow); Ps=38.1

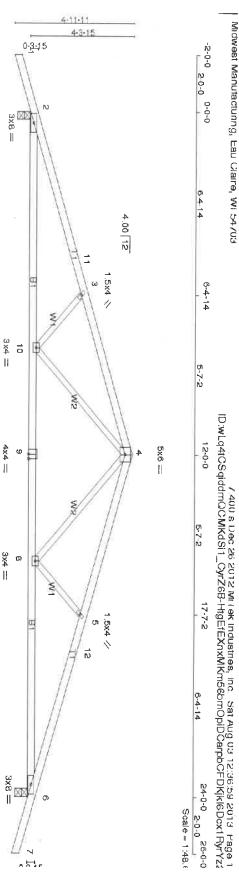
psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.14) Roof design snow load has been reduced to account for slope. Continued on page 2



8-3-4

15-8-12

8-3-4



			90	vuy
REACTIONS (lb/size) 6=1317/0-3-8 Max Horz 2=-61(LC 19) Max Upliff6=-156(LC 11) Max Grav 6=1693(LC 3)	מעבטס באס פון פונום	LUMBER TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WIGHS	BCIL 10.0 *	IING (psf) (roof) (Ps/Pg) 38.1/
(lb/size) 6=1317/0-3-8 (min. 0-2-10), 2=1317/0-3-8 (min. 0-2-10) Max Horz 2=-61(LC 19) Max Upliff6=-156(LC 11), 2=-156(LC 10) Max Grav 6=1693(LC 3), 2=1693(LC 3)			PI2	SPACING 2-0-0 Plates increase 1.15
3-8 (min. 0-2-10)		84 TC	.	CSI TC 0.81
		BRACING TOP CHORD BOT CHORD	Horz(TL)	DEFL Vert(LL)
with Stabilizer Installation guide.	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance	Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing.		(loc) Vdefl 8-10 >999
	s and required cr rection, in accord	applied or 2-2-0 or 0 oc bracing.	Weight: 74 lb	PLATES MT20
	ance	2 purlins.	b FT = 0	GRIP 197/144

JOINT STRESS INDEX 2 = 0.79, 3 = 0.51, 4 = 0.78, 5 = 0.51, 6 = 0.79, 8 = 0.90, 9 = 0.87 and <math>10 = 0.90

FORCES (b) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-11=-3428/328, 3-11=-3240/343, 3-4=-2969/294, 4-5=-2969/294, 5-12=-3240/343, 6-12=-3428/328

BOT CHORD 2-10=-243/3144, 9-10=-121/2160, 8-9=-121/2160, 6-8=-243/3144

WEBS 3-10=-736/156, 4-10=-23/953, 4-8=-24/953, 5-8=-736/156

NOTES

Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DQL=1.60 plate grip DQL=1.60

4) Roof design snow load has been reduced to account for slope Continued on page 2 3) TCLL: ASCE 7-10; Pr=42.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=55.0 psf (ground šnow); Ps=38.1 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.1



Page 3 of 8 7/27/2014

Midwest Manufacturing, Eau Claire, WI 54703

7.400 s Dec 26.2012 Millek Industries, Inc. Sat Aug 03.12:36:59.2013. Page 2. ID:wLq4tCSqiddmQCMKdSi1_CyrZ6B-HtgEfEXnxMKm56bmOpiDCarpbCFDKjkl6Dcx1RyrYz2

NOTES

| Unbalanced snow loads have been considered for this design.
| Unbalanced snow loads have been considered for this design.
| Unbalanced snow loads have been considered for this design.
| This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
| This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
| This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord | No. 1 | No. 2 |

LOAD CASE(S) Standard

and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=156, 2=156.

10) This truss is designed in accordance with the 2012 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Midwest Manutacturing, Eau Claire, WI 54703

7.400 s Dec 26.2012 Millek Industries, Inc. Sat Aug 03.12:36:59.2013. Page 2.10.wLq4tCSqiddmQCMKdSi1_CyrZ6B-HtgEfEXnxMKm56bmOpiDCarpbCFDKjki6Dcx1RyrYz2

NOTES
5) Unbalanced snow loads have been considered for this design.
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 38.1 psf on overhangs non-concurrent with other live loads.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord

and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ji=lb) 6=156, 2=156.

10) This truss is designed in accordance with the 2012 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Page 5 of 8 7/27/2014

Midwest Manutacturing, Eau Claire, WI 54/03 -2-0-0 0-0-0 12-0-0

1.400 s Dec 26 2012 Millek Industries, Inc. Sat Aug 03 12:49:56 2013 Page 1 ID:Fb4XO2_bTeMl6caTScjXnhyMuXZ-1M7_kvw9y5DWpWU5PV8RBQWhrPegtEnfnbKXnvyrYrin

26-0-0

4-11-11 4-3-15 0-3-15 N Ñ U Ŋ σ 19 5x6 == 4×4 || ö 17 9 12-0-0 ā ö - Constant 23 4.00 12 4 3x4 == 24-0-0 2-0-0 Scale = 1:48.6

7/144

FT = 0

REACTIONS All bearings 24-0-0. Max Horz 2=-61(LC 15)

TOP CHORD BOT CHORD

2x4 SPF No.2 2x4 SPF No.2 2x4 SPF Stud

BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

with Stabilizer Installation guide

bracing be installed during truss erection, in accordance MiTek recommends that Stabilizers and required cross TOP CHORD

OTHERS

(B) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14 except 12=100(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 18, 21, 15 except 12=484(LC 3), 2=484(LC 3), 19=310(LC 21), 20=316(LC 21), 22=392(LC 3), 17=310(LC 22), 16=316(LC 22), 14=392(LC 3)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 6-19--270/54, 5-20--274/60, 3-22--332/77, 8-17--270/54, 9-16--274/60,

FORCES

WEBS

JOINT STRESS INDEX
2 = 0.93, 3 = 0.68, 4 = 0.68, 5 = 0.68, 6 = 0.68,
19 = 0.68, 20 = 0.68, 21 = 0.68 and 22 = 0.68 NOTES 7 = 0.46, 8 = 0.68, 9 = 0.68, 10 = 0.68, 11 = 0.68, 12 = 0.93, 14 = 0.68, 15 = 0.68, 16 = 0.68, 17 = 0.68, 18 = 0.22, 18 = 0.88, 1

and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 (Antinued on page 2) 1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left



0-3-15

22

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19

8

17

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4

3x4 II

24-0-0

5x6 🔢

24-0-0

Page 6 of 8 7/27/2014

Midwest Manutacturing, Eau Claire, WI 54703 ID:Fb4XO2_bTeMl6caTSojXnhyMuXZ-1MZ_kvw9y5DWpWU5PV8RBQWhrPegtEnfnbKXnvyrYmv

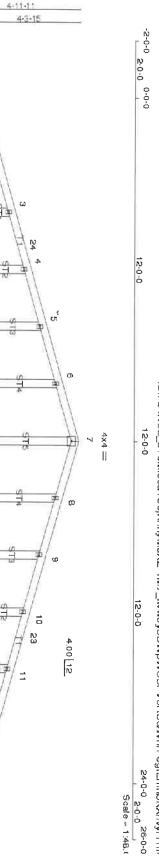


Plate Offsets (X,Y): [18:0-3-0,0-3-0]	0-3-0							
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl L	L/d	PLATES	GRIP
Snow (Ps/Pn) 38 1/55 0	Plates increase 1.15	TC 0.40	Vert(LL)			120		197/144
TCDI 10.0	Lumber Increase 1.15		Vert(TL)	-0.01	n/r	90		
BCI 00 *	Rep Stress Incr YES	WB 0.08	Horz(TL)	0.00 12	_	n/a		
BCDL 10.0	Code IRC2012/TPI2007	(Matrix)					Weight: 85 lb FT = 0	FT = 0
LUMBER TOP CHORD 2x4 SPF No.2		77 12	TOP CHORD	Structural wood	sheathing o	directly apr	wood sheathing directly applied or 6-0-0 or purling	arline.
BOT CHORD 2x4 SPF No.2		B	BOT CHORD		ectly applied	d or 10-0-0	oc bracing.	
OTHERS 2X4 SPF Stud				MiTek recomr	nends that §	Stabilizers :	MiTek recommends that Stabilizers and required cross	SS
				bracing be ins with Stabilizer	talled during Installation	g truss erec guide.	bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	Ce
(lb) - Max Horz 2=-61(LC 15)	24-0-0. (LC 15)							

JOINT STRESS INDEX

WEBS FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 6-19=-270/54, 5-20=-274/60, 3-22=-332/77, 8-17=-270/54, 9-16=-274/60.

Max Uplift All uplift 100 lb or less at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14 except 12=-100(LC 11)

Max Grav All reactions 250 lb or less at joint(s) 18, 21, 15 except 12=484(LC 3), 2=484(LC 3), 19=310(LC 21), 20=316(LC 21), 22=392(LC 3), 17=310(LC 22), 16=316(LC 22), 14=392(LC 3)

NOTES 2 = 0.93, 3 = 0.68, 4 = 0.68, 5 = 0.68, 6 = 0.68, 19 = 0.68, 20 = 0.68, 21 = 0.68 and 22 = 0.68=0.46, 8=0.68, 9=0.68, 10=0.68, 11=0.68, 12=0.93, 14=0.68, 15=0.68, 16=0.68, 17=0.68, 18=0.

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed; end vertical left and right exposed; Cor members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 Confinued on page 2



Page 7 of 8 7/27/2014

Midwest Manufacturing, Eau Claire, WI 54703

1.400 s Dec 26 2012 Mi Lek Industries, Inc. Sat Aug 03 12:49:56 2013 Page 2 ID:Fb4XO2_bTeMl6oaTSojXnhyMuXZ-1MZ_kvw9y5DWpWU5PV8RBQWhrPegtEnfnbKXnvyrYm

NOTES

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP11.

4) TCLL: ASCE 7-10; Pr=42.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=55.0 psf (ground snow); Ps=38.1 psf (roof snow: Lumber DOL=1.15 Plate

DOL=1.15); Category II; Exp B; Fully Exp.; Ct=1.1

5) Roof design snow load has been reduced to account for slope.
6) Unbalanced snow loads have been considered for this design.
7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 38.1 psf on overhangs non-concurrent with other live loads.

8) All plates are 1.5x3 MT20 unless otherwise indicated.9) Gable requires continuous bottom chord bearing.

10) Gable studs spaced at 2-0-0 oc

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14 except (it=lb) chord and any other members.

14) This truss is designed in accordance with the 2012 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP11

LOAD CASE(S) Standard



Page 8 of 8 7/27/2014

Midwest Manutactunng, Eau Claire, WI 54703

7.400 s Dec 26 2012 Mi Fek Industries, Inc. Sat Aug 03 12:49:56 2013 Page 2 ID:Fb4XO2_bTeMi6oaTSojXnhyMuXZ-1M7_kvw9y5DWpWU5PV8RBQWhrPegtEnfinbKXnvyrYrm

NOTES

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP11.

4) TCLL: ASCE 7-10; Pr=42.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=55.0 psf (ground snow); Ps=38.1 psf (roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Fully Exp.; Cl=1.1

5) Roof design snow load has been reduced to account for slope.
6) Unbalanced snow loads have been considered for this design.
7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 38.1 psf on overhangs non-concurrent with other live loads.
7) All plates are 1.5x3 MT20 unless otherwise indicated.
9) Gable requires continuous bottom chord bearing.

10) Gable studs spaced at 2-0-0 oc.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.12) "This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 19, 20, 21, 22, 17, 16, 15, 14 except (ji=lb)

14) This truss is designed in accordance with the 2012 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1

LOAD CASE(S) Standard

